





James D. Rotunno
Structural Option

The Butler Health System's – New Inpatient Tower Addition

AE Senior Thesis
April 13th, 2010
The Pennsylvania State University



Building for the Future: A New Era Begins



Presentation Outline:

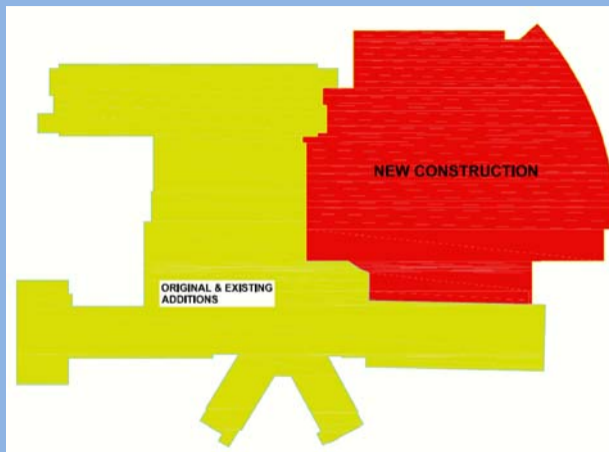
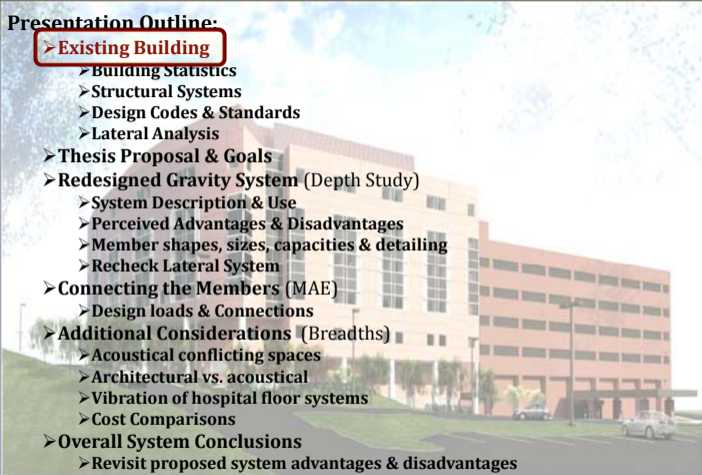
- **Existing Building**
 - Structural Systems
 - Design Codes & Standards
 - Lateral Analysis
- **Thesis Proposal & Goals**
- **Redesigned Gravity System (Depth Study)**
 - System Description & Use
 - Perceived Advantages & Disadvantages
 - Member shapes, sizes, capacities & detailing
 - Recheck Lateral System
- **Connecting the Members (MAE)**
- **Additional Considerations (Breadths)**
 - Acoustical for conflicting spaces
 - Architectural vs. acoustical
 - Vibration of hospital floor systems
 - Cost Comparisons
- **Overall System Conclusions**
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 - Design limitations
- **Acknowledgments**

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Project Team:

Owner: Butler Healthcare Providers

Owners Representative: Ritter Const. Management Inc.

Construction Manager: Turner Construction

Architect: Design Group

Design Architect: Hammel, Green, Abrahamson HGA

Size: 206,000 Square Feet

Height: 134'-3" from lowest level

Levels: 6 Above Grade & 2 Below Grade

Construction Dates: September 2008 – Summer 2010

Function: Primarily Surgery & Recovery

Cost: \$93M (GMP)



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Existing Structural Systems:

- Steel Wide Flange Members for Columns & Beams
- HSS Members for Various Braced Frame Configurations
- Floor Systems are Composed of Composite Deck & Composite Beams



Drilled piers 30"-78" Ø;
down to 79' deep



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Design Standards & Codes:

2006 IBC

2000 NFPA 101

2006 Guidelines for Design & Construction of Health Care Facilities

1998 Pennsylvania Department of Health Rules and Regulations for Hospitals

ASCE 7-05: for wind, seismic, snow and gravity loads

ACI 318-08: for concrete construction

AISC Thirteenth Edition: for steel members

ASHRAE Handbook: HVAC Applications & Fundamentals 2003

PCI 2003 for vibration

ATC 1999 for vibration (ADAPT technical note TN209 3/21/09 for reference)

Design Load Summary:

Gravity Loads					
Description/location	DL/LL	ASCE 7-05/IBC 1607.9 values	HGA's values	Reduction available/used	Design value
Concrete floors	DL	90-115pcf	115pcf	NO/NO	115pcf
MEP/partitions/finishes	SDL	20-25psf	44psf	NO/NO	35psf
1 st floor mechanical	LL		125psf	YES/NO	125psf
2 nd floor/ lobby	LL	100psf	100psf	YES/NO	100psf
Hospital floors	LL	40-80psf	80psf	YES/YES	80psf
Stairs & exits	LL	100psf	100psf	NO/NO	100psf
5 th floor roof	LL		115psf	NO/NO	115psf
Mech. Penthouse floor	LL		125psf	NO/NO	125psf
Elevator Machine room floor	LL		125psf	YES/NO	
Roof top equipment areas	LL		125psf (or actual equipment wt.)	NO/NO	125psf
Balconies	LL	100psf	100psf	YES/YES	100psf
Snow	LL	24-30psf	24-30psf	NO/NO	24-30psf

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Wind Load Data for Calculations

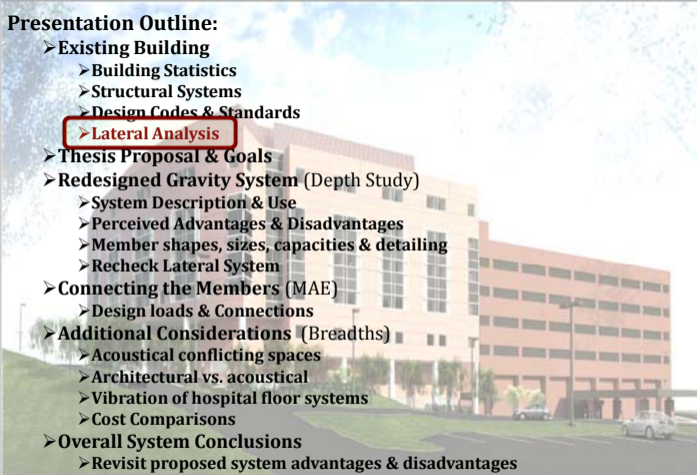
North-South direction			ASCE section
Basic wind speed	V	90mph	6.5.4 (Figure 6-1)
Mean roof height	h	122ft	
Wind directionality factor	K_d	0.85	6.5.4 (Table 6-4)
Importance Factor	I	1.15	6.5.5 (Table 6-1)
Exposure category		C	6.5.6.3
Velocity pressure coefficient	K_z	varies	6.5.6 (Table 6-3)
Topographic factor	K_{zt}	varies	6.5.7 (Figure 6-4)
Gust effect factor	G	0.857	6.5.8
Enclosure Classification		Enclosed	6.5.9
Internal pressure coefficient	GC_{pi}	± 0.18	6.5.11.1 (Table 6-3)
External pressure coefficients windward side	C_p	0.8	6.5.11.2 (Figure 6-6)
External pressure coefficients leeward side	C_p	-0.5	(Figure 6-6)
Velocity pressure @ height Z	q_z	varies	6.5.10
Velocity pressure @ mean roof height	q_h	30.41/ft ²	6.5.10
Design wind load	F	determine	

North - South Base & Story Shears with Overturning Moment

Level	Height ft	Pressure lbs/ft ²	Force(F) kips	Shear (V) kips	Moment (M) Kips*ft
		Windward + leeward			
0- Ground	0	0	0	557.55	4086.84
1	14'-8"	24.60	15.69	557.55	4086.84
2	29'-4"	26.61	72.10	541.86	3971.83
3	44'-0"	27.33	98.45	469.76	3443.34
5	58'-8"	27.61	100.27	371.31	2721.70
6	73'-4"	27.63	93.73	271.04	1986.72
7	88'-0"	27.43	86.37	177.31	1299.68
8-Roof	102'-8"	26.91	62.53	90.94	666.59
9- P.H. 1	122'-0"	26.34	23.96	28.41	274.58
10- P.H. 2	135'- 0"	25.90	4.45	4.45	28.93
Base Shear =				557.55	
Overturning Moment =					22567.05

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Deflection criteria as per 2006 International Building Code:

Allowable building drift: $\Delta_{wind} = H/400$

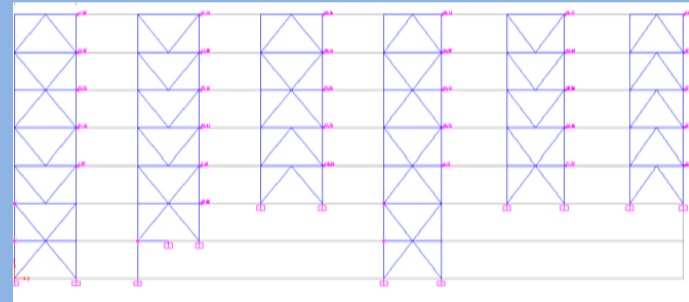
Allowable story drift: $\Delta_{seismic} = 0.10h_{sx}$ (Table 12.12-1 ASCE 7-05)

Equation used to calculate story drift Δ_s : $K=P/\Delta_p$ $\Delta_p=P/K$

Wind Drift Comparison of Frame #2

Level	Story Height (ft)	Story Drift (in)	Allowable Story Drift $\Delta_{wind} = H/400$ (in)	Total Drift (in)	Allowable Total Drift $\Delta_{wind} = H/400$ (in)	
3	14.67	0.0782	< 0.44	Acceptable	0.0782 < 1.32	Acceptable
5	14.67	0.0837	< 0.44	Acceptable	0.162 < 1.76	Acceptable
6	14.67	0.0782	< 0.44	Acceptable	0.240 < 2.2	Acceptable
7	14.67	0.0856	< 0.44	Acceptable	0.326 < 2.64	Acceptable
8/roof	14.67	0.0620	< 0.44	Acceptable	0.388 < 3.08	Acceptable

Concentric Braced Frame Configurations



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- **Replace the existing gravity system with a modified version of the Girder-slab D-beam System**
- **Check lateral system compatibility & loading**
- **Check connections for feasibility**
- **Acoustical considerations of conflicting spaces**
- **Vibration considerations**
- **Structural System cost analysis estimate**

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➤ Reinforce structural classes by studying a relatively new structural system concept and modifying the idea to expand its current use.

➤ What are the defining limits and parameters keeping this system from expanding its use?

➤ Can the use of the proposed system be expanded to this type of structure or others?

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GIRDER SLAB
COMPOSITE STEEL AND PRECAST SYSTEM

RECIPIENT OF
AISC SPECIAL
ACHIEVEMENT
AWARD

GIRDER-SLAB
TECHNOLOGIES, LLC

- THE GROUTING PROCESS IS EASILY PERFORMED WITH A FEW TRADESMEN. THE CEMENT GROUT IS LIQUEFIED AND PUMPED THROUGH A HOSE. WORKERS PUDDLE THE GROUT IN ORDER TO FILL IN THE VOIDS AND SLAB CORES.
- UNLIKE CAST-IN-PLACE CONCRETE STRUCTURES, THE GIRDER-SLAB SYSTEM IS ASSEMBLED-IN-PLACE.
- THE UNDERSIDE OF SLAB IS READY MADE FOR CEILING FINISH.
- THE INNOVATIVE D-BEAM GIRDER WAS DESIGNED TO ALLOW THE PRECAST SLAB TO SET ON ITS BOTTOM FLANGE CONCEALING ITS TOP FLANGE AND WEB. NO FORMWORK OR SHORING IS NEEDED.
- ALLOWS FASTER ACCESS FOR THE WORK OF OTHER TRADES. CORING OF SLABS FOR UTILITIES IS EASIER AND PERMITS FINAL ADJUSTMENT.
- AFTER GROUTING, THE SLAB IS COMPLETE AND READY FOR USE. FINISH FLOOR PREPARATION WORK CAN TAKE PLACE BEFORE OR AFTER INTERIOR WALLS.
- PRECAST SLABS CAN BE SET IN PLACE IN NEARLY ANY CLIMATE CONDITION INCLUDING FREEZING TEMPERATURES.
- AFTER SLABS ARE SET, GROUT IS EASILY PLACED FLOWING AROUND THE D-BEAM AND THROUGH ITS TRAPEZOIDAL SHAPE WEB OPENINGS AND INTO THE SLAB CORES.
- THE UNDERSIDE OF SLAB IS FREE OF SUPPORT BEAMS PROVIDING A FLAT SURFACE FOR DUCTS AND PIPING SYSTEMS. MINIMUM CEILING HEIGHTS OF 8'-0" ARE EASILY ATTAINED.

HOTELS
STUDENT HOUSING
CONDOS & APARTMENTS
TALL BUILDINGS



Project: [Marriott Courtyard](#)



Project: [Lawrenceville Graduate Apartments at Princeton University](#)

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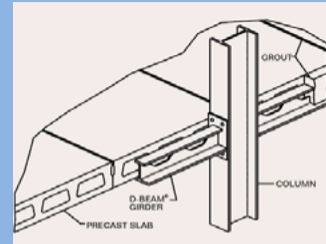
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Disadvantages:

- ❖ Large lead times with this type of system
- ❖ Girders and columns would need fireproofing
- ❖ Much more efficient and cost effective at shorter spans
- ❖ Column spacing may have to be reduced, increasing footing requirements
- ❖ Floor penetrations must be well coordinated with the slab designer/manufacture

Advantages:

- ❖ Easy & fast to install
- ❖ The lateral system can still be utilized
- ❖ No formwork required and concrete slabs are already at usable capacity when they arrive
- ❖ No intermediate beams in interior of bays needed
- ❖ Can be installed in any type of weather
- ❖ Other trades can start work underneath almost immediately
- ❖ Additional unobstructed ceiling space for MEP's.
- ❖ Meets or exceeds floor fireproofing requirements
- ❖ Reduce noise transmission from floor to floor through baffled cavities
- ❖ No increase in floor to floor heights
- ❖ Reduces overall weight of the structure



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Required Moments vs. Spans

Span (ft)	M_u @ 80psf LL & Constant DL (k*ft)	M_u @ 125psf LL & Constant DL (k*ft)
14	207	260
27	770	967
28	828	1040
29	888	1115
30	950	1193
31	1014	1274
32	1080	1358

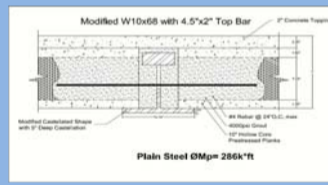
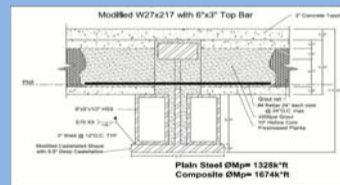
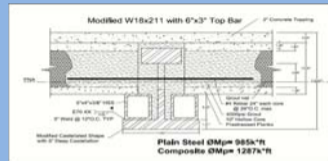
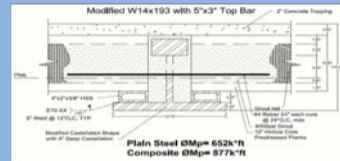
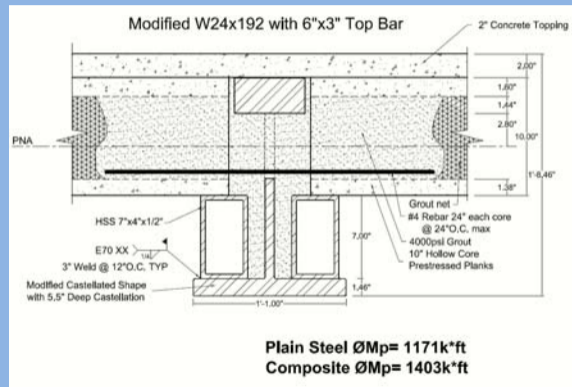
Shear & Moment Capacity per Shape

Modified Girder Shape Size (Modified)	Shear Capacity @ Least Section (kips)	Total Depth Inc. 2" Concrete Topping (in)	Non-composite Plastic Moment Capacity (ΦM_p) (k*ft)	Composite Plastic Moment Capacity (ΦM_{pc}) (k*ft)
W _m 27x217	359.8	22.50	1328	1674
W _m 24x192	314.0	20.46	1171	1403
W _m 18x211	345.0	18.91	985	1287
W _m 14x193	233.6	15.44	652	877
W _m 10x68	70.1	12	286	Uncalculated

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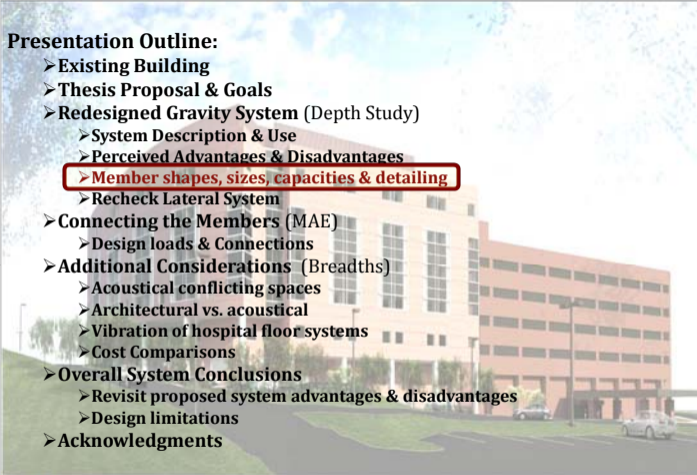
Modified D-Beam Girder Shapes



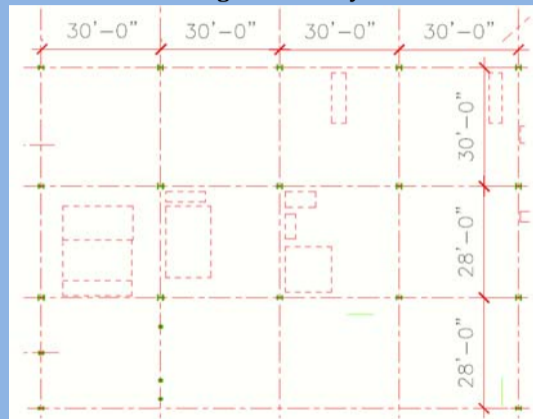
Other Designed Shapes

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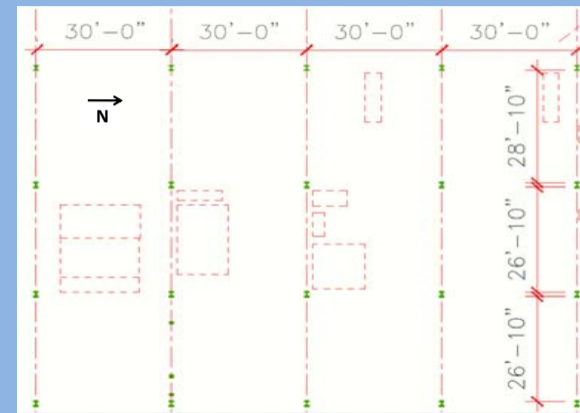
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As Designed Floor System

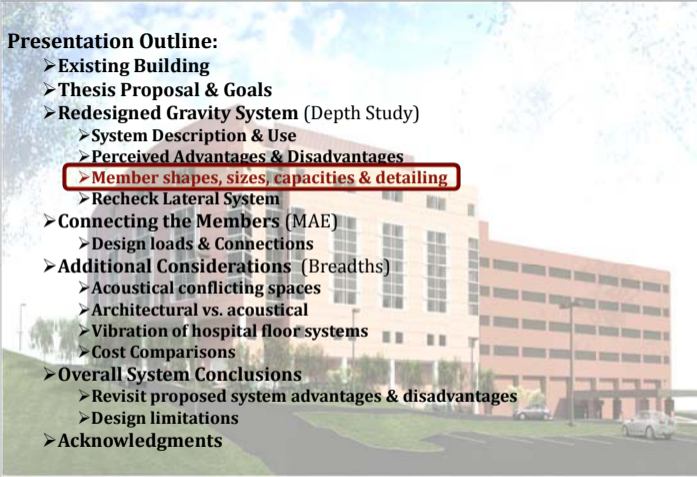


Proposed Floor System

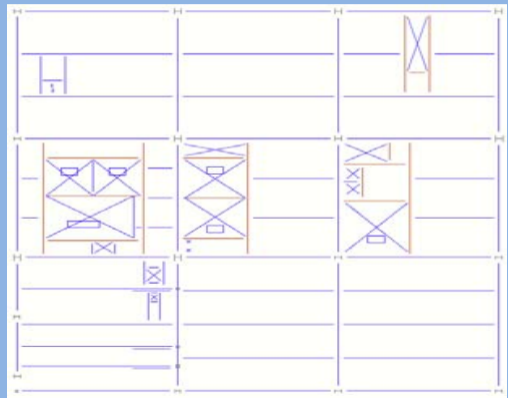


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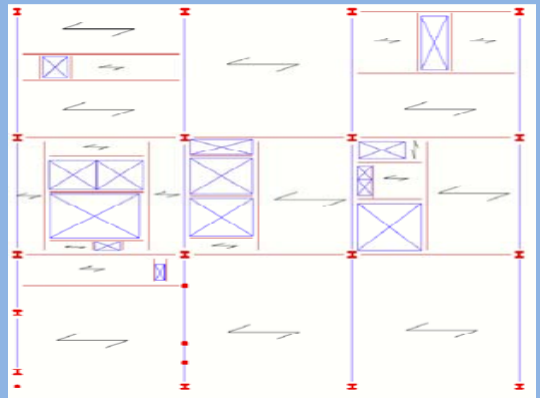
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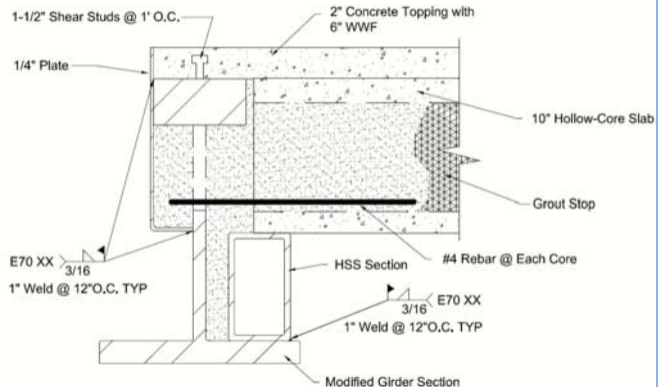
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Detail Around Openings



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Total Dead Load for Seismic Calculation

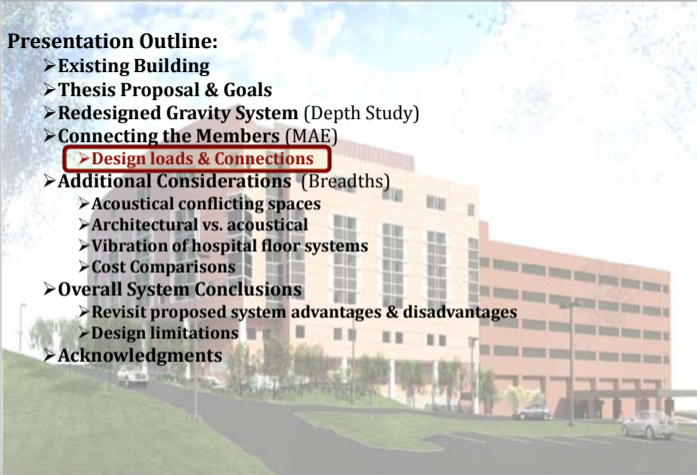
Floor Level	W _T Load type									Floor weight Totals W _T	F _x kips
	square footage	wall square footage	Plank & Topping psf	Superimposed MEP/Partition s	Columns kips	Beams lb/ft ²	equipment psf	roof psf	exterior walls psf/wall		
Ground	8240		93.0	35.0		10.0	1.0	93.0	28.6		
Level 1	20405	170	1897.67	714.18	70.07	204.05	20.41	0	4.86	2906.4	29.06
Level 2	45545	458	4235.69	1594.08	60.70	455.45	45.55	0	13.10	6391.5	63.91
Level 3	42165	458	3921.35	1475.78	82.79	421.65	42.17	0	13.10	5943.7	59.44
Level 5	31525	458	2931.83	1103.38	50.20	315.25	31.53	0	13.10	4432.2	44.32
Level 6	27720	678	2577.96	970.20	47.40	277.20	27.72	0	19.39	3900.5	39.00
Level 7	27760	678	2581.68	971.60	35.83	277.60	27.76	0	19.39	3894.5	38.94
Level 8 (roof)	45545							4235.69		4235.7	42.36
TOTALS	248905	2900	18146.16	6829.2	346.99	1951.2	195.12	4235.7	82.94		

$$W_T = 31787.3 \text{ kips}$$

$$\text{Base Shear} = 317.04 \text{ kips}$$

Presentation Outline:

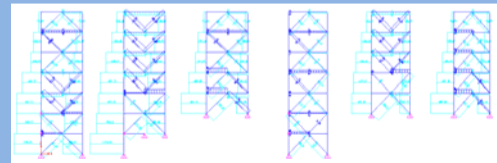
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Full gravity and calculated lateral loads for connection designs

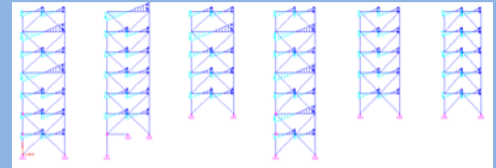


Axial Loads generated



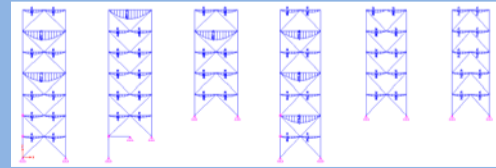
 Compressive  Tensile

Shears generated



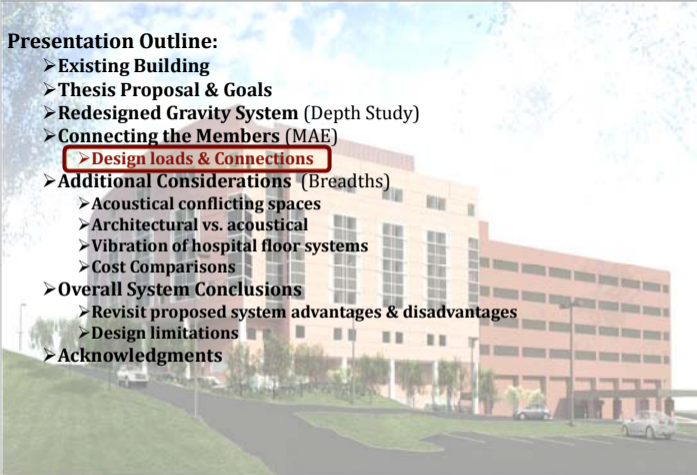
Positive  Negative 

Moments generated

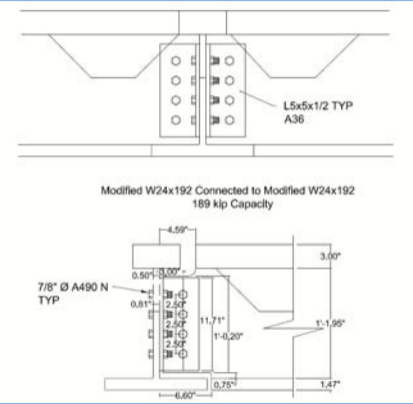


Presentation Outline:

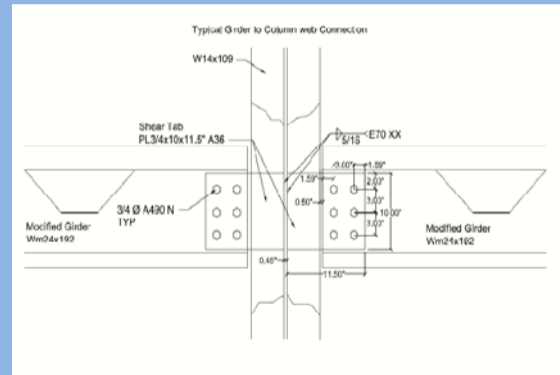
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Double Angle Modified Girder to Modified Girder

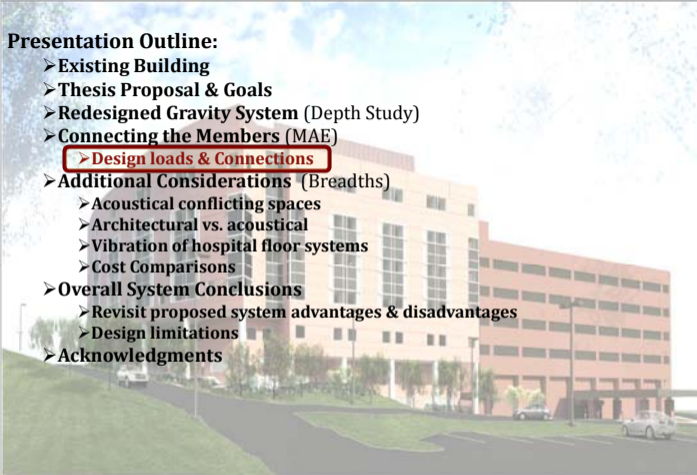


Extended Shear Tab Modified Girder to Column Web

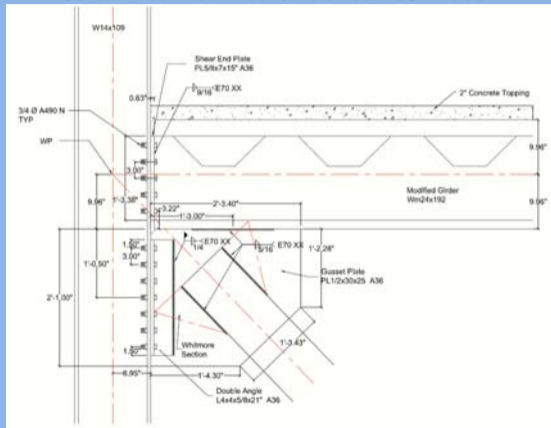


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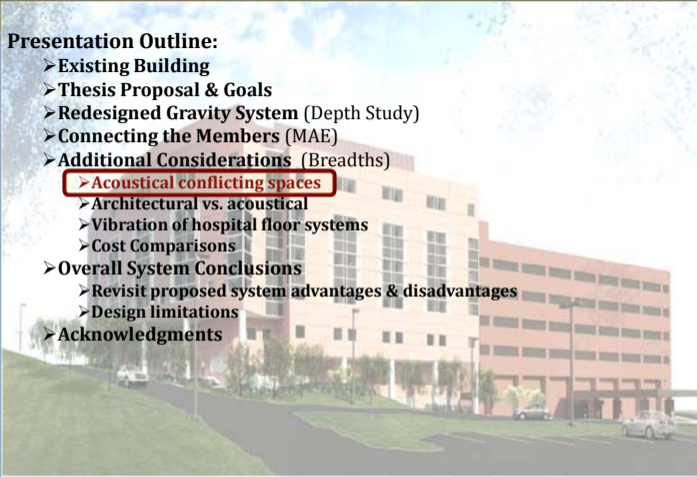


Column to Modified Girder to HSS Brace



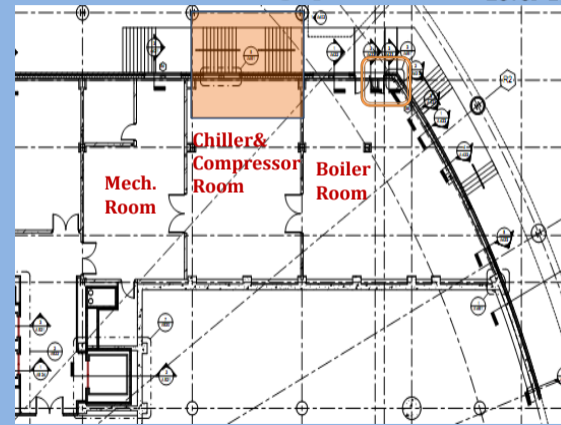
Presentation Outline:

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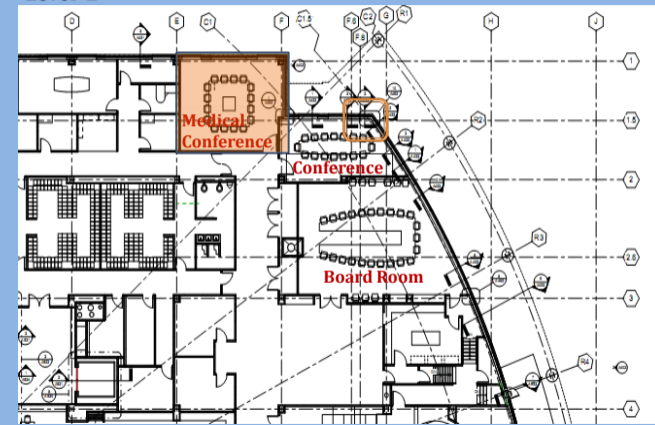


Conflicting Spaces

Level 1

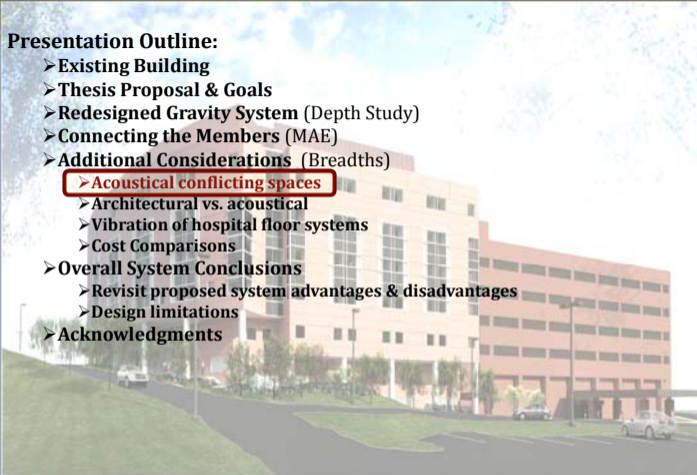


Level 2



Presentation Outline:

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Building for the Future: A New Era Begins

Receiver Room Sound Correction As Designed

Hz	63	125	250	500	1000	2000	4000	8000
Max. dB	80	75	92	88	90	87	79	67
Build up	+9	+9	+9	+9	+9	+9	+9	+9
total	89	84	101	97	99	96	88	76
A weighting	-25	-15	-8	-3	+0	+1	+1	+1
A weighted adjusted	64	69	93	94	99	97	89	75
TOTAL (dBA)	64	70	93	95	100	102	102	102

Floor Systems Effectiveness Comparison

As Designed	Proposed
$25 \geq 105 - 51 = 54$	$25 \geq 105 - 57 = 48$
25 < 54 NOT ACCEPTABLE	25 < 48 NOT ACCEPTABLE

Presentation Outline:

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TOTAL (dBA)	64	70	93	95	100	102	102	102

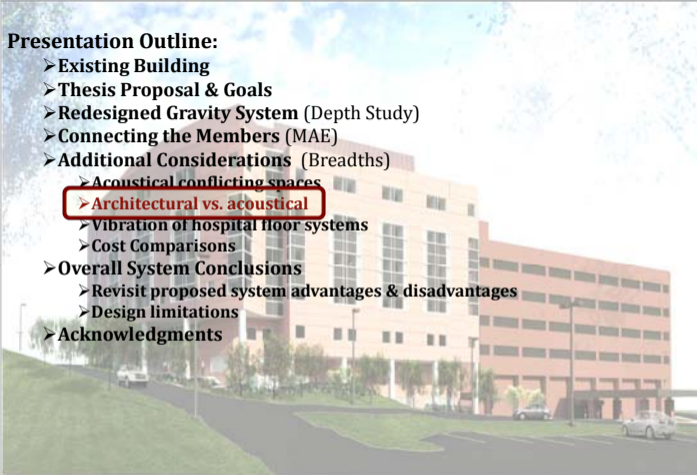
Floor Systems Effectiveness Comparison	
As Designed	Proposed
25 ≥ 105-51=54	25 ≥ 105-57=48
25 < 54 NOT ACCEPTABLE	25 < 48 NOT ACCEPTABLE

Receiver Room Sound Correction with Acoustical Barrier								
Hz	63	125	250	500	1000	2000	4000	8000
Max. dB	80	75	92	88	90	87	79	67
Build up	+6	+6	+6	+6	+6	+6	+6	+6
total (+A)	86	81	98	94	96	93	85	73
(+B)	+1	+0	-1	-2	-3	-4	-5	-5
total (+B)	-9	-9	-9	-9	-9	-9	-9	-9
total Art composite TL	78	72	88	83	84	80	71	59
Art composite TL	-	-10	-12	-16	-21	-26	-32	-
total	-	62	76	67	63	58	39	-
A weighting	-25	-15	-8	-3	+0	+1	+1	+1
A weighted adjusted	-	47	68	64	63	59	40	-
TOTAL (dBA)	-	47	68	69	70	70	70	70

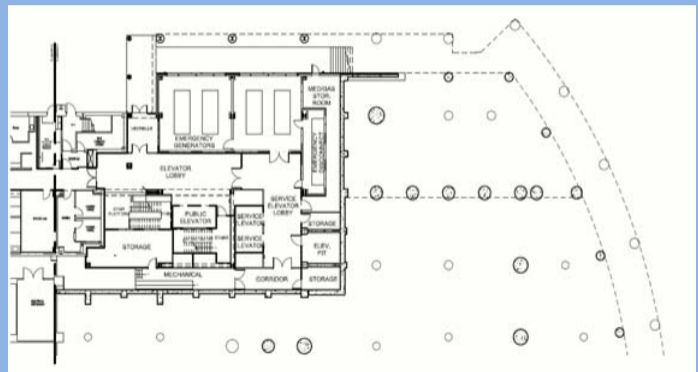
Floor Systems Effectiveness Comparison with Sound Barrier	
As Designed	Proposed
25 ≥ 73-51=22	25 ≥ 73-57=16
25 > 22 ACCEPTABLE	25 > 16 ACCEPTABLE

Presentation Outline:

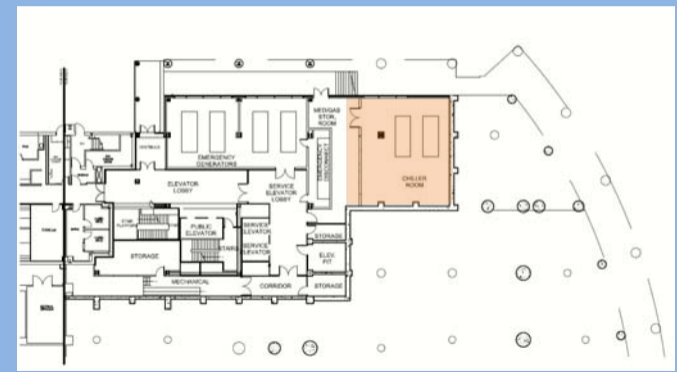
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As Designed Ground Level

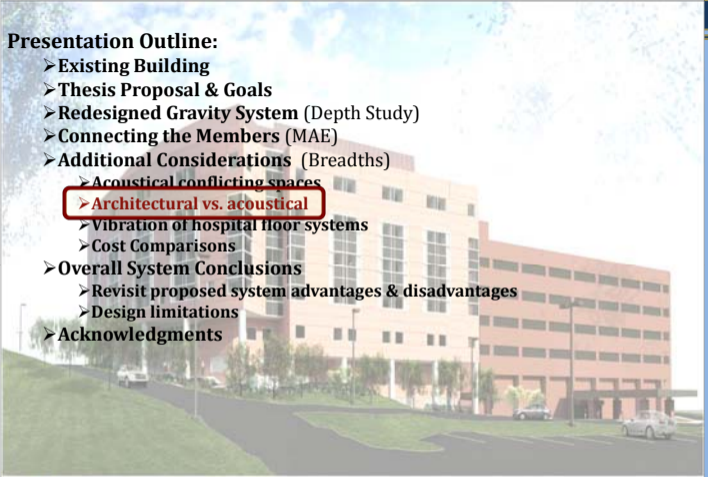


Proposed Ground Level

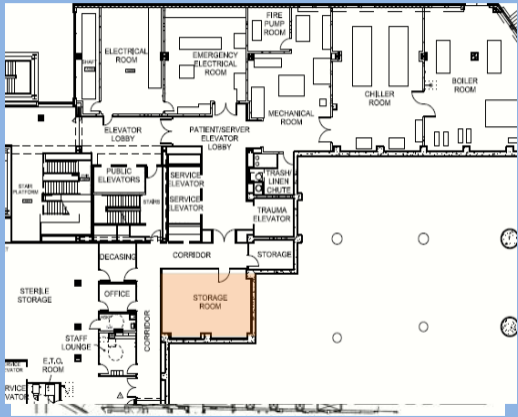


Presentation Outline:

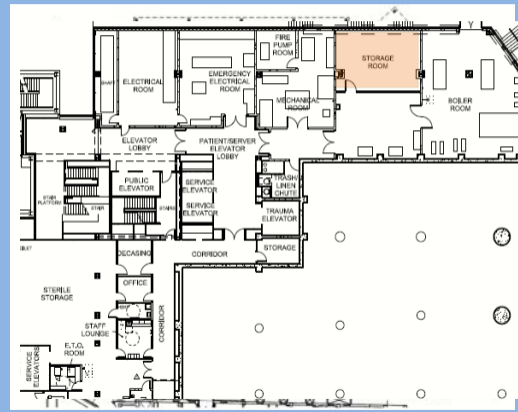
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As Designed Level 1

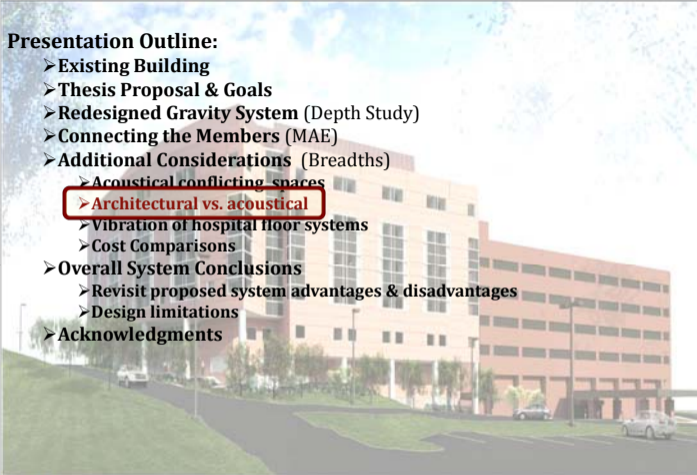


Proposed Level 1



Presentation Outline:

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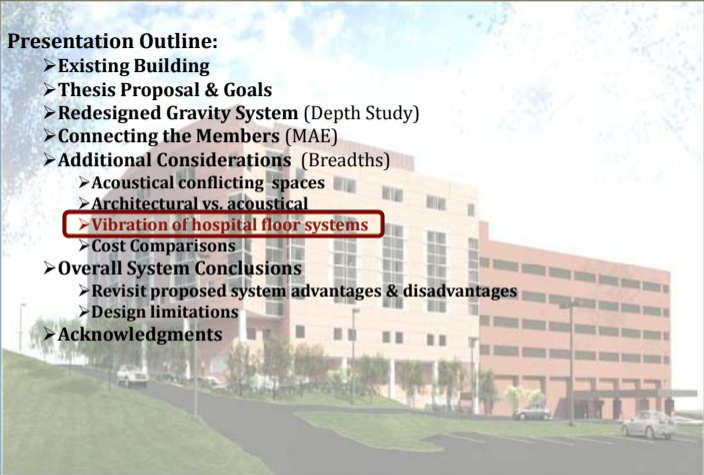


Acoustical Treatment VS. Architectural Redesign

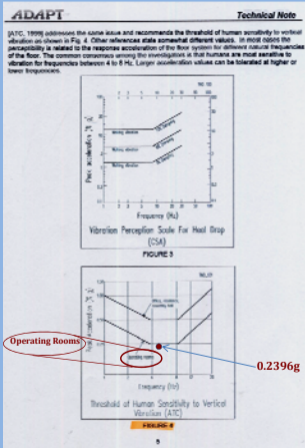
Acoustical Considerations	Estimated Cost (\$)	Redesign Considerations	Estimated Cost (\$)
Sound Barrier	7,500.00	Excavation of 8400ft ³	1440.00
Adhesive	450.00	Additional 60' of Foundation Walls (Ground)	25,645.00
Labor	15,840.00	Additional 44' of 8" Reinforced CMU Wall	4818.00
		Additional Slab On Grade	9800.00
		Less 5 Columns @15'	-6263.00
		Additional 2 sets of double doors	6000.00
		Additional 30' of interior wall for storage area	1200.00
		Less 54' of Foundation Wall (1 st)	-23,528.00
		Mechanical Considerations (pipes, ducts, sprinkler)	3500.00
TOTAL	23,790.00	TOTAL	22,612.00

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Building for the Future: A New Era Begins



Floor Excitation Analysis

Sources: PCI & ADAPT

Assumptions:

P_o = weight of an individual = 150lb
 β = 0.05 (TN209)

Calculated Values:

W = weight of the floor section = 107.5k
 f_n = fundamental natural frequency = 5.2Hz

$$a = [P_o e^{(-0.35 \cdot f_n)} / \beta W] g \leq 0.25\% g$$

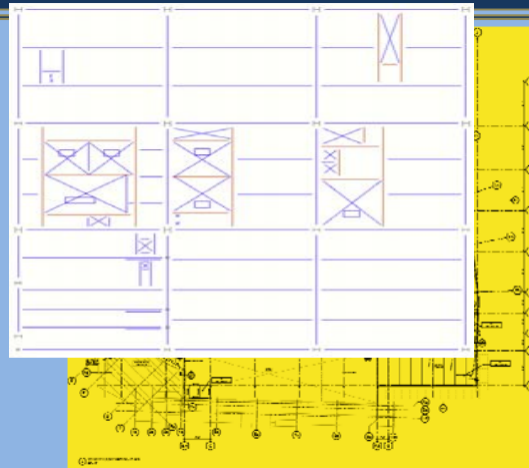
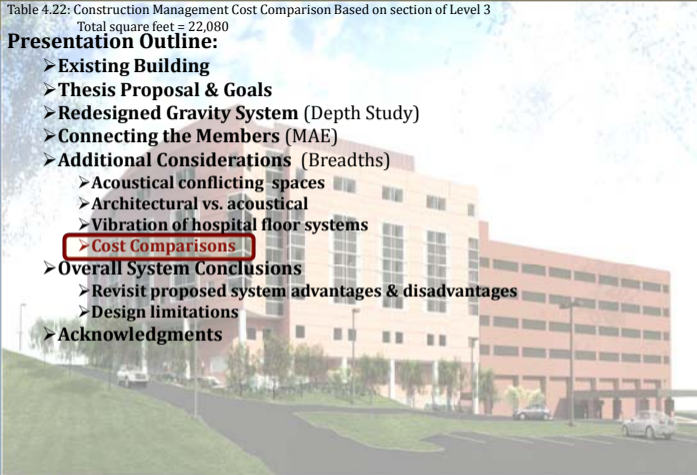
$$a = 0.2396g \quad \text{OK}$$

Table 4.22: Construction Management Cost Comparison Based on section of Level 3

Total square feet = 22,080

Presentation Outline:

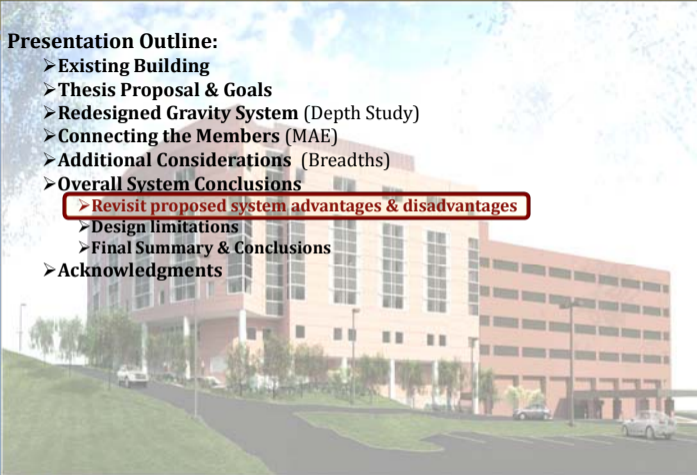
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Cost Comparison of Structural Systems			
As Designed	Estimated Cost (\$)	Proposed	Estimated Cost (\$)
		Licensing Fee	206,000
Columns (42 @ 69kips)	3500	Columns (42 @ 82.8kips)	3500
Labor to install		Labor to install	
Fabrication	411,337	Fabrication	1,319,640
Girders (37 @ 45.7kips)	3500	Girders (37 @ 203.3kips)	3500
Labor to install		Labor to install	
Beams (121 @ 102.7kips)	10,500	Beams (40 @ 116.2kips)	3500
Labor to install		Labor to install	
Connections (336)	252,000	Connections (142)	106,500
Shear studs & decking (2177) & (22,080ft ²)	135,667	Shear studs (175)	347
Concrete forming & placement 3pours @ 7360ft ² ea.	155,142	Hollow-core slab & install	234,048
		Opening & grouting HCS	44,160
		2" Concrete topping	34,707
Fireproofing (200 full members)(Total feet=4649)	19,850	Fireproofing (119 members) (Total feet=2150)	5850
TOTAL	991,400	TOTAL	1,755,800
		DIFFERENCE	764,400

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Disadvantages:

- ❖ Large lead times with this type of system
- ❖ Girders and columns would need fireproofing
- ❖ Much more efficient and cost effective at shorter spans
- ❖ Column spacing may have to be reduced, increasing footing requirements
- ❖ Floor penetrations must be well coordinated with the slab designer/manufacture

Advantages:

- ❖ **Easy & fast to install**
- ❖ The lateral system can still be utilized
- ❖ No formwork required and concrete slabs are already at usable capacity when they arrive
- ❖ **No intermediate beams in interior of bays needed**
- ❖ **Can be installed in any type of weather**
- ❖ **Other trades can start work underneath almost immediately**
- ❖ Additional unobstructed ceiling space for MEP's.
- ❖ Meets or exceeds floor fireproofing requirements
- ❖ Reduce noise transmission from floor to floor through baffled cavities
- ❖ No increase in floor to floor heights
- ❖ **Reduces overall weight of the structure**

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Limiting Factors

Large Opening Sizes

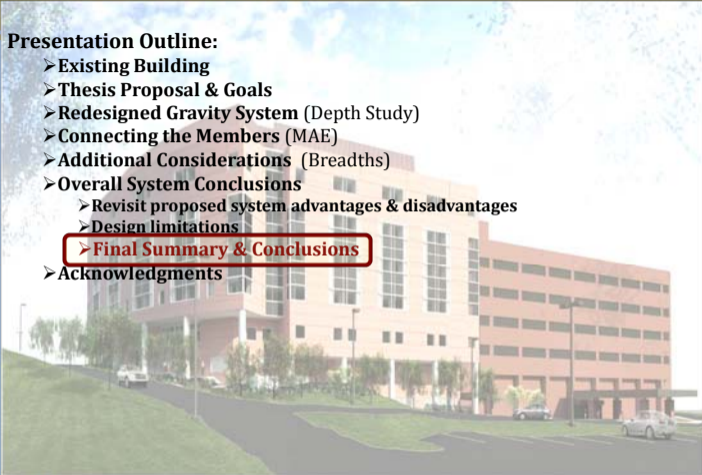
Dimension Between Top Bar & Bottom Flange

Hollow-core Plank Span



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Use of Proposed System in a Hospital Building

- Large openings required
- Increased structural costs
- Extra space in ceiling cavity desired (not reduced)
- System inflexibility

Theoretically possible; NOT PROBABLE

Use of Proposed System in Other Structure Types

- Can reduce floor to floor depths by 1' - 1.5'
 - Without reducing open unobstructed ceiling cavity space
 - Reduces costs associated with façade, elevators, stairs, MEP runs, column lengths and sizes, bracing length and sizes, interior partition wall heights, fireproofing, Heating & cooling costs.
- Better acoustical and vibration aspects

POSSIBLE & PROBABLE



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- **Acknowledgements**



Butler Health System: Owner

Turner Construction: Construction Manager
Kurt Johnson – Project Manager
William Beck – Project Superintendent
Megan Wortman – Field Engineer

HGA: Design Architect, Structural, Mechanical, Electrical Engineer
Johanna H. Harris P.E. – Associate Vice President
Jonathan Wacker

Girder-Slab: Daniel G. Fisher Sr. – Managing Partner
Peter Naccarato P.E. – Engineer

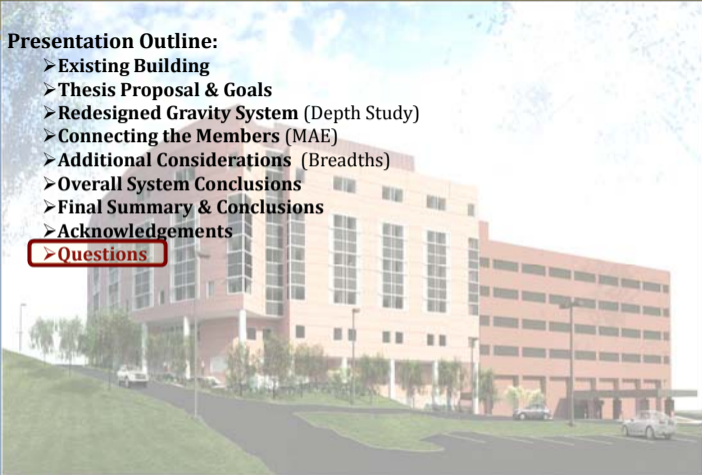
Pennsylvania State University: Department of Architectural Engineering
Dr. Ali Memari, Ph.D., P.E., – Thesis Advisor
Louis F. Geschwindner Jr., Ph.D., P.E.
Professor Emeritus
AISC Vice President, Special Projects
M. Kevin Parfitt, P.E. – Thesis Faculty Director
Faculty & Staff

Family & Friends



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- **Questions**



Questions / Comments



